

### A methodology for the bivariate hydrological characterisation of the overtopping failure for river levees

Matteo Isola, Enrica Caporali and Luis Garrote

matteo.isola@unifi.it



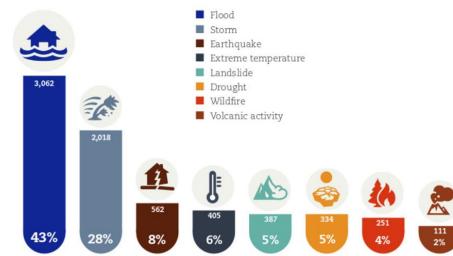


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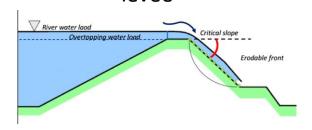
### FRAMEWORK



Percentage of occurrences of natural disasters by disasters type (1995-2015)

# AIM

Improve the current practice of the overtopping risk analysis for a river levee



Several from the most flood disaster are caused by Levee Failure



### OVERTOPPING IS THE MAIN CAUSE OF LEVEE FAILURE (Vorogushyn et al. 2010)



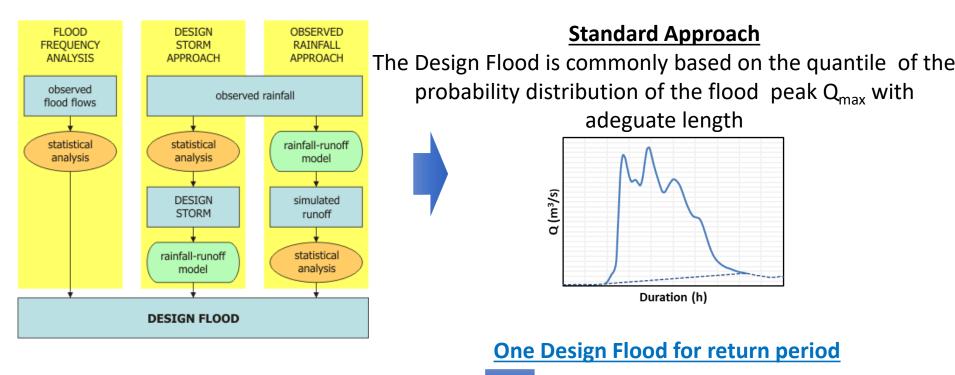


A METHODOLOGY FOR THE BIVARIATE HYDROLOGICAL CHARACTERIZATION OF FLOOD WAVES FOR RIVER-RELATED FLOOD RISKS ASSESSMENTS

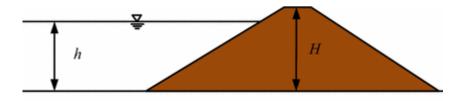
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## RIVER FLOOD RISK ANALYSIS: CURRENT APPROACH



Stage in the river depends by the entire hydrograph

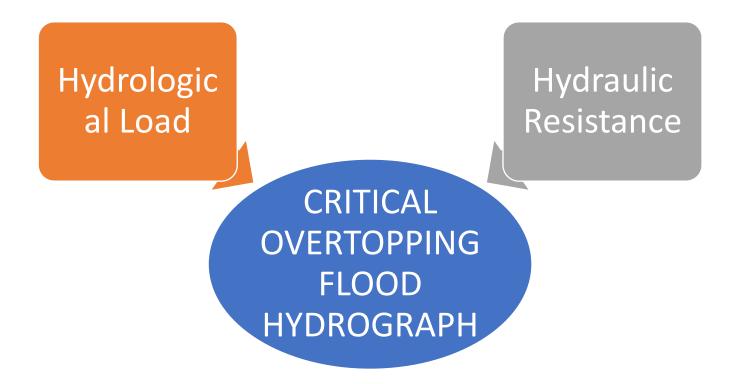


BIVARIATE ANALYSIS OF PEAK DISCHARGE (Q) AND VOLUME OF HYDROGRAPH (V)





# METHODOLOGY

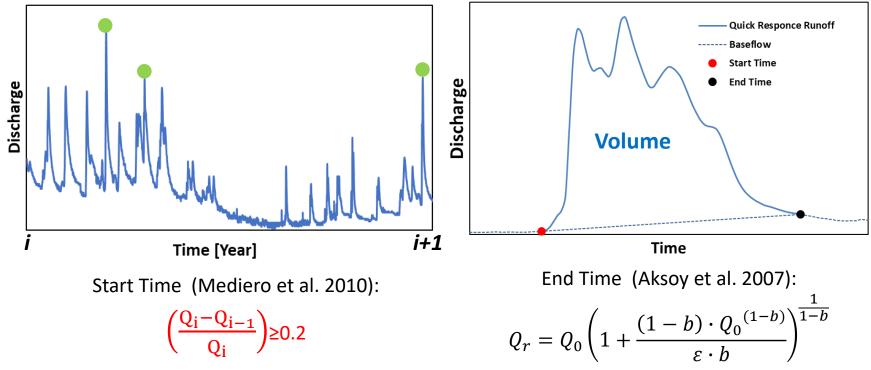




# **METHODOLOGY**

### HYDROLOGICAL LOAD

1. Detection of observed Peak Discharges and Volumes series (Peak Over Threshold)



Generation of Synthetic Peak (Q) through inverse of Generalized Pareto Distribution 2. (GPD) and parameters estimation through L-moments method.

GPD 
$$(x|u,\beta,\gamma,\mu) = P(X > x|X > u)1 - \left[1 + \frac{\gamma \cdot (x-\mu)}{\beta}\right]^{-1/\gamma}$$

 $\beta = \text{scale}, \gamma = \text{shape and } \mu = \text{location}$ 





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# METHODOLOGY

### HYDROLOGICAL LOAD

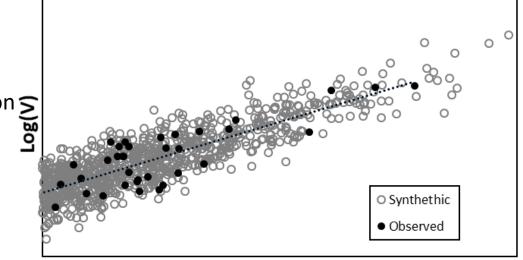
Normal Randomization to generate synthetic Volume (V)

$$V = \frac{1}{\sigma\sqrt{2\pi}} e^{-0.5\left(\frac{x-\mu}{\sigma}\right)}$$

With Mean ( $\mu$ ) equal the Regression Equation between  $Q_{obs}$  and  $V_{obs}$ 

$$\mu = k \cdot Q_{synth}^{a}$$

With Standard deviation ( $\sigma$ ) equal Residual Variance of the Regression



Log(Q)

$$\sigma = \sqrt{\frac{\sum_{i=1}^{n} \left(\log(V_{obs}) - \log(\log(k \cdot Q_{obs}^{\alpha}))\right)^2}{n-1}}$$



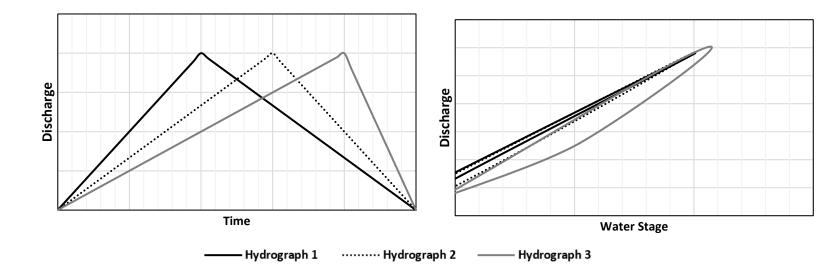


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# METHODOLOGY

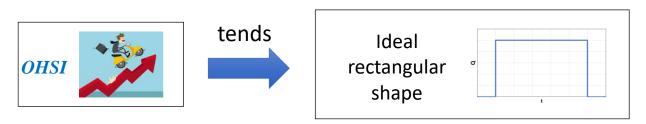
### HYDROLOGICAL LOAD

### SHAPE



Shape Classification through Overtopping Hydrograph Shape Index (OHSI) (Isola et al. 2020)

$$OHSI = \left(\frac{X_{rl,ad} \cdot D_r + X_{fl,ad} \cdot D_f}{D}\right) \cdot \left(\frac{Y_{rl,ad} \cdot (Q_p - Q_{min,rl}) + Y_{fl,ad} \cdot (Q_p - Q_{min,fl})}{(Q_p - Q_{min,rl}) + (Q_p - Q_{min,fl})}\right)$$



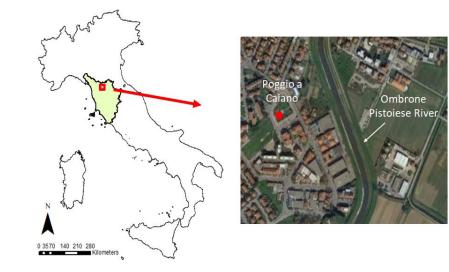




# CASE STUDY

We tested the methodology **for two case studies** (ex. Ombrone river, Era River), here we show results about of Ombrone Pistoiese River located in the town of Poggio a Caiano. The Ombrone Pistoiese river is a tributary of Arno River in Tuscany, central Italy.

- The catchment area:  $\simeq$ 450 km<sup>2</sup>
- Hydrometric Gauge station named "Ponte all' Asse"
- Collection of hourly series of discharge data for the period 1992-2016





The river reach is completely banked

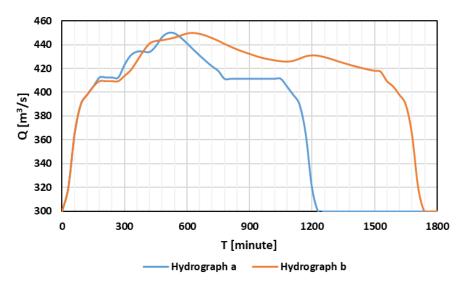
### Test the methodology OVERTOPPING FAILURE & FLOOD DAMAGE ESTIMATION





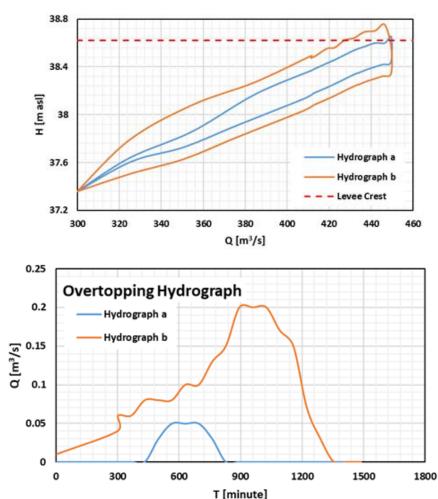
### EVALUATION OF THE HYDROGRAPHS

Peak discharge and Volume from approximated bivariate distribution Shape: observed shape classified through OHSI and sensitivity analysis



H-b exceeds the levee resistance criterion conditions and cause the overtopping failure.

<u>Levee resistance criterion</u> <u>10000 m<sup>3</sup> per m for 2 hours</u> <u>(Damme et al., 2016) .</u>





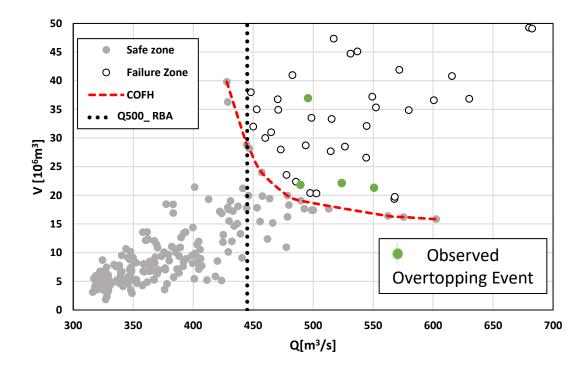


# CRITICAL OVERTOPPING FLOOD HYDROGRAPH

The **Critical Overtopping Flood Hydrograph** is a curve in peak-volume space which contains every hydrograph that cause the **overtopping failure threshold condition**.

#### **OVERTOPPING LEVEE RESISTANCE CONDITION**

- The water level exceeds the levee crest in one or more cross-sections of the river.
- The overtopping volume exceeds 10000 m<sup>3</sup> per m for 2 hours (Damme et al., 2016).



Poggio a Caiano 1992 Flood for levee overtopping



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For further information about the work, please see the article:

Journal of Hydrologic Engineering

# River Levee Overtopping: A Bivariate Methodology for Hydrological Characterization of Overtopping Failure

🛅 Matteo Isola; 🛅 Enrica Caporali; and 🛅 Luis Garrote

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# THANKS FOR YOUR READING

